

2. (a) liquid B because it is a shorter column than A

(b) pressures - at pt. ①: $p = 0$ (atmospheric)
pt. ②: $p = \rho_{Hg} g (h_1 - h_2) = \rho_w g (h_3 - h_2)$
pt. ③: $p = 0$

4. Continuity and Bernoulli Eq. application

$$\begin{aligned} a) \quad v_1 &= 2 \text{ m/s} & D_1 &= 8 \text{ cm} \Rightarrow A_1 = \pi (0.04 \text{ m})^2 = 0.0016\pi \text{ m}^2 \\ v_2 &=? & D_2 &= 3 \text{ cm} \Rightarrow A_2 = \pi (0.015 \text{ m})^2 = 0.000225\pi \text{ m}^2 \\ v_3 &=? & D_3 &= 6 \text{ cm} \Rightarrow A_3 = 0.0009\pi \text{ m}^2 \end{aligned}$$

$$Q = v_1 A_1 = (2 \text{ m/s}) (0.0016\pi \text{ m}^2) = 0.01 \pi \text{ m}^3/\text{s}$$

$$v_2 = \frac{0.01 \text{ m}^3/\text{s}}{0.000225\pi \text{ m}^2} = \underline{14.3 \text{ m/s}}$$

$$v_3 = \frac{0.01}{0.0009\pi} = \underline{3.57 \text{ m/s}}$$

b) taking $z=0$ at the pipe centerline and noting that $1 \text{ psi} \approx 70 \text{ cm of H}_2\text{O}$

$$\begin{aligned} H_1 &= \frac{P_1}{\gamma} + z_1 + \frac{v_1^2}{2g} = 3(70 \text{ cm H}_2\text{O}) + 0 + \frac{4 \text{ m}^2/\text{s}^2}{2(9.8)} \\ &= 2.1 \text{ m} + 0.2 \text{ m} = 2.3 \text{ m} \end{aligned}$$

$$H_3 = H_1 - H_L = 2.3 \text{ m} - 0.5 \text{ m} = 1.8 \text{ m}$$

$$\begin{aligned} &= \frac{P_3}{\gamma} + \frac{v_3^2}{2g} \quad \text{or} \quad \frac{P_3}{\gamma} = 1.8 \text{ m} - \frac{(3.57)^2}{2(9.8)} = 1.16 \text{ m} \\ &= \underline{116 \text{ cm of H}_2\text{O}} \end{aligned}$$

6. The driving force behind the flow is the head loss between the two reservoirs, that is, $h_f = 80 - 65 = 15 \text{ m}$. Assuming turbulent flow

$$h_f = f \frac{L}{D} \frac{v^2}{2g} = 15 \text{ m}$$

$$v = \sqrt{\frac{(0.12 \text{ m})(2)(9.81)(15)}{(0.025)(460)}} = 1.75 \text{ m/s}$$

$$Q = vA = 1.75 \pi (0.06 \text{ m})^2 = 0.020 \text{ m}^3/\text{s} = \underline{20 \text{ l/sec}}$$

8. $H_1 + H_p = H_2$; $Q = 28 \text{ l/sec} = 0.028 \text{ m}^3/\text{s}$

$$h_1 + \frac{v_1^2}{2g} + H_p = h_2 + \frac{v_2^2}{2g}$$

$$\downarrow \qquad \qquad \qquad \downarrow$$

$$30 \text{ m} \qquad \qquad \qquad 60 \text{ m}$$

The manometers indicate a piezometric pressure head because they are referenced to some elevation datum not specified in the problem.
 $D_1 = 0.15 \text{ m}$, $D_2 = 0.10 \text{ m}$

$$v_1 = \frac{0.028 \text{ m}^3/\text{s}}{\pi (0.075 \text{ m})^2} = 1.58 \text{ m/sec}$$

$$v_2 = \frac{0.028 \text{ m}^3/\text{s}}{\pi (0.05)^2} = 3.565 \text{ m/s}$$

$$H_p = (60 - 30) + \frac{1}{2g}(v_2^2 - v_1^2) = 30 \text{ m} + \frac{1}{2g}(10.2) = 30.52 \text{ m}$$

$$P = H_p Q \gamma = (30.52 \text{ m})(0.028 \frac{\text{m}^3}{\text{s}})(9800 \frac{\text{N}}{\text{m}^3})$$

$$= \underline{\underline{8.375 \text{ kW}}}$$

10 Determine the flowrate if $P = 40 \text{ kW}$ rather than 15.55 as above

$$H_p = H_2 - H_1 = (z_2 - z_1) + (\frac{f_2}{\gamma} - \frac{f_1}{\gamma}) + \frac{1}{2g}(v_2^2 - v_1^2)$$
 ; $v = \frac{Q}{A}$

$$= (21 - 0) + (0 - 6) + \frac{Q^2}{2g} (\frac{1}{A_2^2} - \frac{1}{A_1^2}) = 15 + Q^2 \frac{48034}{2g} = H_p$$

$$P = 40000 \text{ W} = [15 + 2450.7 Q^2] Q \gamma \text{ or } 15Q + 2450.7 Q^3 = \frac{40000}{9800}$$

$$2450.7 Q^3 + 15Q = 4.08$$
 trial & error solution for Q

$$\frac{P}{\gamma}(Q) = 600.46 Q^3 + 3.676 Q = 4$$
 ; $Q \approx \underline{\underline{0.102 \text{ m}^3/\text{sec}}}$

Increasing the pump energy did little for increasing the flowrate!

12 Basically, this question is asking for the upstream pressure head. $Q = 0.085 \frac{\text{m}^3}{\text{s}}$
 $D_1 = 0.3 \text{ m}$
 $D_2 = 0.1 \text{ m}$

$$H_1 = h + 0 + \frac{v_1^2}{2g}$$
 } $H_1 = H_2 + H_{\text{turbine}} = H_2 + \frac{15000}{(0.085)(9800)}$ $v_1 = 1.2 \text{ m/s}$

$$H_2 = 0 + 0 + \frac{v_2^2}{2g}$$
 } $\downarrow 18 \text{ m}$ $v_2 = 10.82 \text{ m/s}$

$$h = 18 \text{ m} + \frac{1}{2g}(v_2^2 - v_1^2) = 18 + \frac{1}{2g}(115.7) = \underline{\underline{23.9 \text{ m}}}$$

14 The slope of the pipe is the same as the head loss per meter, or h_f/L .
 Assuming turbulent flow again.

$$\frac{h_f}{L} = \frac{f}{D} \frac{v^2}{2g}$$
 and $v = \frac{Q}{A} = \frac{200 \text{ l/s}}{\pi (0.5 \text{ m})^2} \cdot \frac{\text{m}^3}{1000 \text{ l}} = 0.255 \text{ m/s}$

$f \approx 0.018$ (Moody Diagram)

$\frac{\epsilon}{D} = \frac{0.36 \text{ mm}}{1000 \text{ mm}} = 0.00036$

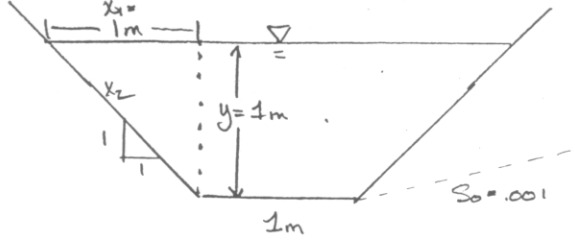
$R = \frac{vD}{\nu} = \frac{(0.255 \frac{\text{m}}{\text{s}})(1 \text{ m})}{1.31 \times 10^{-6} \frac{\text{m}^2}{\text{s}}} = 194388 \approx 2 \times 10^5$

$$\frac{h_f}{L} = \text{slope} = \frac{0.018}{1} \cdot \frac{(0.255)^2}{2(9.8)} = 0.00006 \text{ or } \underline{\underline{0.006\%}}$$

Q	f(Q)
0.1	0.968
0.11	1.204
0.105	1.081
0.102	1.012

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$Q = ?$
 $n = 0.024$



$Q = AC \sqrt{RS}$ $C = R^{1/6}/n$ $R = \frac{A}{W_p}$

① Area: $1m^2 + \frac{1}{2}(1m)(1m) \times 2$ * we know the ht. (y)
 $= 2m^2$ b/c the side slopes are 1:1, we know x₁

② $W_p = 1m + 2(1.4m)$ $x_2 = \frac{1}{\sin 45^\circ}$
 $= 3.8m$

③ R (hydraulic radius) = $\frac{2m^2}{3.8m} = 0.526$ $0.5224 = R$

④ $C = \frac{0.5224^{1/6}}{0.024} = 37.4$ $C = \frac{0.5224^{1/6}}{0.024} = \frac{.8974}{0.024} = 37.3$

$So, Q = (2m^2) (37.4) (0.5224) (.001)^{1/2} = 1.72 m^3/s$ $Q = (2m^2)(37.4)(0.5224)(.001)$
 note that units don't work out

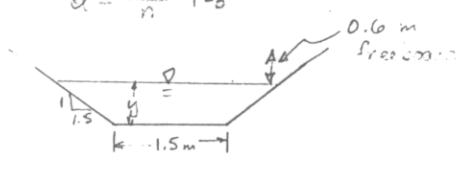
~~$1.664 m^3/s$~~

$Q = 1.709 m^3/s$

20. Open-channel problem $Q = AC \sqrt{RS_0}$ and $C = \frac{R^{1/6}}{n}$

$Q = 200 \text{ l/sec} = 0.2 m^3/\text{sec}$
 $S = 0.001$
 $n = 0.04$

$Q = \frac{AR^{2/3}}{n} \sqrt{S_0}$



$A = \frac{1}{2} [1.5m + (1.5 + 2(1.5y))] y$
 $w_p = 1.5 + 2(1.5y^2 + y^2)^{1/2} = 1.5 + \sqrt{1.5} y$
 $= 1.5y + 1.5y^2$

$R = \frac{A}{w_p} = \frac{1.5(y+y^2)}{1.5 + \sqrt{1.5} y}$
 $= \frac{y+y^2}{1+0.82y}$

$AR^{2/3} = Qn/\sqrt{S_0} = (0.2)(0.04)/(0.001)^{1/2} = 0.253$

$f(y) = 1.5(y+y^2) \left[\frac{y+y^2}{1+0.82y} \right]^{2/3} = 0.253$ requires a trial & error solution

y	f(y)
1	3.2
0.3	0.27
0.4	0.47
0.28	0.236

$3.0 \left(\frac{2}{1.8} \right)^{2/3}$
 $y = 0.29 m$

- (a) water depth = 29 cm
- (b) Top width = $1.5 + 3(.87) = \underline{4.17 m}$
- (c) water surface width = 2.37 m

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Force balance problem similar to class notes - convert to SI units for ease

$$F + (P_1 A_1 - P_2 A_2) = \rho Q (v_2 - v_1) \quad \text{where } D_1 = 1 \text{ in} = 0.0254 \text{ m} \\ D_2 = \frac{1}{4} \text{ in} = 0.0063 \text{ m}$$

$$Q = 20 \frac{\text{gal}}{\text{min}} \cdot \frac{\text{m}^3}{264.2 \text{ gal}} = 0.073 \text{ m}^3/\text{min}$$

Absolute pressures are necessary (1 atm = 14.7 psi = 1030 cm of H₂O)

$$P_1 = 45 + 14.7 \text{ psi} = 59.7 \text{ psi} \cdot \frac{0.704 \text{ m}}{\text{psi}} \cdot \frac{9800 \text{ N}}{\text{m}^2} = 411.9 \text{ kPa}$$

$$P_2 = 14.7 \text{ psi} = 14.7 (0.704)(9800) = 101.4 \text{ kPa}$$

$$(P_1 A_1 - P_2 A_2) = (411.9 \text{ kPa})(0.0254 \text{ m})^2 \cdot \frac{\pi}{4} - (101.4)(0.0063)^2 \cdot \frac{\pi}{4} \\ = 208.7 \text{ N} - 3.16 \text{ N} \\ = 205.5 \text{ N}$$

$$\rho Q (v_2 - v_1) = (1000 \frac{\text{kg}}{\text{m}^3})(0.073 \frac{\text{m}^3}{\text{min}})(\frac{\text{min}}{60 \text{ s}})(v_2 - v_1) = 44.5 \text{ N}$$

$$v_1 = \frac{Q}{A_1} = \frac{0.073 \text{ m}^3/\text{min}}{\frac{\pi}{4} (0.0254 \text{ m})^2} \cdot \frac{\text{min}}{60 \text{ sec}} = 2.4 \text{ m/s}$$

$$v_2 = \frac{0.073}{\frac{\pi}{4} (0.0063)^2} \cdot \frac{1}{60} = 39.07 \text{ m/s}$$

$$(v_2 - v_1) = 36.6 \frac{\text{m}}{\text{s}}$$

$$\therefore F = 44.5 \text{ N} - 205.5 \text{ N} = \underline{\underline{-161 \text{ N}}} \quad (\text{the minus sign indicates } \leftarrow F)$$